GENERAL NOTES

SCOPE:

NEW EXTERIOR ELEVATOR AND STAIRS INCLUDING RETAINING WALLS. NEW EXTERIOR ACCESS RAMP, RELOCATION OF A NEW INTERIOR ELEVATOR AND INFILL EXISTING FLOOR SHAFT, NEW OPERABLE PARTITION, NEW OPENINGS IN FLOORS, AND INFILL OF OPENINGS IN FLOORS.

GENERAL:

- PERFORM THE WORK IN THE ORDER INDICATED ON THE DRAWINGS WHERE WORK REQUIRES SEQUENTIAL OPERATIONS.
- WHEN REQUESTING SUBSTITUTIONS FOR PRODUCTS, PROCEDURES, METHODS OR MATERIALS SPECIFIED FOR THE PROJECT, SUBMIT ENGINEERING DATA ESTABLISHING EQUIVALENCE AND ICC ACCEPTANCE NUMBER, IF APPLICABLE, FOR REVIEW BY THE ENGINEER AND APPROVAL BY DSA PRIOR TO INCORPORATING INTO THE WORK.
- WHERE REFERENCED INDUSTRY STANDARDS ARE LISTED, USE THE LATEST DSA ACCEPTED AND APPROVED EDITION.
- THESE DRAWINGS HAVE BEEN CREATED BASED ON THE FOLLOWING DESIGN CRITERIA:

	DESIG	N CRIT	ERIA					
	Gl	ENERAL						
CODE	CODE		2007 CALIFORNIA BUILDING CODE [CBC]					
JURISDICTION		DIVISION	DIVISION OF STATE ARCHITECTS					
OCCUPANCY CATEGORY		111	III .					
LOWEST ANTICIPATED SERVICE TEMP		NA	NA					
LIVE LOAD		EARTHQ	EARTHQUAKE LOAD					
ROOF	20 PSF	SITE LO	SITE LOCATION		LONGITUDE 37.447487			
2ND FLOOR, TYP.	50 PSF			LATTITUDE -	-122.266231			
	PSF	DESIGN	CATEGORY	Ε				
	PSF	Ss	(SEE NOTE)	2.331	1.8648			
WIND LOAD		SD1	(SEE NOTE)	1.191	0.9528			
BASIC WIND SPEED	85 MPH	SITE CL	ASS	D				
EXPOSURE	С	SDS	(SEE NOTE)	1.554	1.2430			
IMPORTANCE FACTOR	1.25	SDL	(SEE NOTE)	0.794	0.6352			
INTERNAL PRESSURE	+/18	R		5.0				
COMPONENTS/CLADDING	10 PSF	1		1.25				
SNOW LOAD		ρ		NA				
Pf	NA	OVERST	OVERSTRENGTH FACTOR		2.5			
Се	NA	ANALYSI	ANALYSIS PROCEDURE		FORCE			
IMPORTANCE FACTOR	NA	BASIC R	RESISTING SYSTEM					
Ct	NA	Rp		2.5				
		lp		1.0				

LEFT NUMBER PER AISC 7-05. SECT. 11.4.5 RIGHT NUMBER- SITE SPECIFIC VALUES PER GEOTECH LETTER, REPORT/P.3 DATED DECEMBER 11, 2008

DIMENSIONS AND DATUM:

- DIMENSIONS ARE GIVEN TO CENTERLINE OF COLUMNS AND BEAMS OR FACE OF WALLS AND ROUGH CONCRETE SURFACES, UNLESS OTHERWISE
- ELEVATIONS ARE GIVEN WITH REFERENCE TO EXISTING FINISHED GROUND FLOOR ELEVATION: EL. = 0'-0", UON. DATUM ELEVATION = +659'-0"

EXISTING CONSTRUCTION:

- DETAILS AND DIMENSIONS FOR EXISTING CONSTRUCTION HAVE BEEN TAKEN FROM DRAWINGS. VERIFY THAT THE EXISTING CONDITIONS AFFECTED BY THE WORK ARE AS INDICATED ON THE DRAWINGS AND, IN CASE OF DISCREPANCY, PROCEED ONLY AFTER RECEIVING INSTRUCTIONS FROM THE ENGINEER.
- VERIFY THAT DISCONNECTING, REMOVING OR DEMOLISHING ANY EXISTING CONSTRUCTION WILL NOT CAUSE INSTABILITY IN ADJACENT CONSTRUCTION PRIOR TO COMMENCING WORK. SHOULD CONDITIONS BECOME EVIDENT WHICH COULD POSE A DANGER TO THE SAFETY OF THE BUILDING, WORKPLACE, WORKERS OR OCCUPANTS, NOTIFY THE OWNER'S REPRESENTATIVE.
- WHERE NEW PENETRATIONS ARE TO BE CUT INTO EXISTING CONCRETE, DRILL OR CORE HOLES AT THE CORNERS OF NEW PENETRATIONS AND SAWCUT BETWEEN HOLES. DO NOT OVERCUT BEYOND THE LIMITS OF THE NEW PENETRATION. IF NECESSARY, REMOVE THE REMAINDER TO SQUARE CORNERS TAKING CARE NOT TO DAMAGE PARTS TO REMAIN.
- WHERE DETAIL NOTES INDICATED THAT EXISTING REINFORCING BARS SHALL NOT BE DAMAGED DURING DEMOLITION, EXISTING CONCRETE SHALL BE DEMOLISHED WITH CARE SUCH THAT EXISTING EMBEDDED CONCRETE REINFORCING SHALL NOT BE CUT AND SHALL REMAIN IN PLACE TO BE PART OF NEW CONSTRUCTION.

METAL STUDS AND JOISTS:

- COMPLY WITH THE PROVISIONS OF CBC CHAPTER 22A AND AISI "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" AND STEEL STUD MANUFACTURERS ASSOCIATION ICC ER-4943P.
- MATERIALS:

GALVANIZED STUDS: ASTM A653, STRUCTURAL STEEL 68 & 54 MIL.: GRADE 50 OR GRADE 33 ASTM A653, STRUCTURAL STEEL GRADE 33 43 MIL. AND THINNER:

CARBON STEEL STUDS: ASTM A1011, STRUCTURAL STEEL 68 & 54 MIL.:

GRADE 50 OR 33 43 MIL. AND THINNER: ASTM A1011, STRUCTURAL STEEL GRADE 33 SUBMIT MANUFACTURER'S DATA, INCLUDING ICC ACCEPTANCE REPORT,

INDICATING COMPLIANCE WITH SECTION PROPERTIES LISTED ON THE

WHERE FASTENING OF MEMBERS IS NOT OTHERWISE INDICATED ON THE

DRAWINGS, FASTEN AS FOLLOWS:

STUDS TO TRACK, EACH FLANGE: 1-#8 SCREW OR SPOT WELD JOIST TO TRACK

JOIST TO STUD WEB TO WEB: 3-#8 SCREWS USE 54 MIL. x 12" BLOCKING w/ 3-#8 TOP TRACK SPLICES SCREWS AT EACH FLANGE, EACH SIDE OF SPLICE

INSTALL CONTINUOUS END BLOCKING AT JOISTS ON TOP PLATES. SOLID BLOCKS, SAME GA. AS STUDS. INSTALL BLOCKING AND DRAFT STOPS AT 10'-0"o.c. MAX. BETWEEN ALL

INSTALL LATERAL BRACING OF BOTH VERTICAL AND HORIZONTAL MEMBERS

WHERE INTERIOR STUD SIZES ARE NOT OTHERWISE INDICATED ON THE DRAWINGS, SPACE THE STUDS AT 16"o.c. AND USE THE FOLLOWING SSMA

UNSUPPORTED LENGTH LESS THAN 12'-0" STUD SIZE & THICKNESS 362S125-43 12'-0" TO LESS THAN 24'-0" $\Delta = 1/240$ 600S125-33

WHERE HEADER SIZES ARE NOT OTHERWISE INDICATED ON THE DRAWINGS, USE THE FOLLOWING:

LESS THAN 6'-0" (2) - 362S125-54 AND (2) - 362T125-54 6'-0" TO 10'-0" (2) - 6000S125-54 AND (2) - 600T125-54 DÉTAIL CONNECTION PER 9.1M, 9.1G & 9.1L -

ALL MEMBERS ARE TO BE CONTINUOUS, WITHOUT SPLICES, BETWEEN SUPPORT POINTS, UNLESS SHOWN OTHERWISE ON THE DRAWINGS.

- COMPLY WITH THE PROVISIONS OF CBC CHAPTER 18A.
- WHERE PRACTICABLE, MAKE EXCAVATIONS AS NEAR AS POSSIBLE TO THE NEAT LINES REQUIRED BY THE SIZE AND SHAPE OF THE STRUCTURE. EXCAVATE NO MATERIAL UNNECESSARILY.
- WHERE EXCAVATIONS CANNOT BE MAINTAINED FOR A NEAT POUR, FORM THE SIDES. WHEN POURING FOOTINGS NEAT, ADD ONE INCH EACH SIDE OF FOOTINGS TO THE SIZES SHOWN ON THE DRAWINGS.
- ELEVATIONS OF BOTTOMS OF FOOTINGS, HAVE BEEN ESTABLISHED TO REACH COMPETENT NATURAL SOILS OR ENGINEERED FILL AS DETERMINED FROM GEOTECHNICAL INVESTIGATION REPORT BY CONRNERSTONE EARTH GROUP, DATED MARCH 20, 2008 AND AMENDED DECEMBER 11, 2008. THIS MATERIAL IS CAPABLE OF SUPPORTING ALLOWABLE LOADS AS INDICATED BELOW:

BASED ON SOIL OR FILL BASED ON COMPETENT ROCK 2,000 PSF 6,000 PSF DEAD LOAD + LIVE LOAD 3,000 PSF 9,000 PSF TOTAL LOAD INCL. SEISMIC 4,000 PSF 12,000 PSF

AS EXCAVATION PROGRESSES, CONDITIONS MAY DEVELOP REQUIRING CHANGES IN ELEVATIONS OF FOOTINGS, SHOWN ON THE DRAWINGS. MAKE SUCH CHANGES ONLY AS DIRECTED BY THE ENGINEER.

CLEAN EXCAVATIONS JUST PRIOR TO PLACING CONCRETE.

WHERE BACKFILL IS TO BE PLACED AGAINST WALLS BEFORE THEY HAVE ATTAINED THEIR DESIGN STRENGTH, SHORE THE WALLS TO SUPPORT THE SOIL LOADING. THE SHORING IS TO REMAIN IN PLACE UNTIL THE WALL HAS ATTAINED ITS DESIGN STRENGTH AND/OR OTHER CONSTRUCTION INTENDED TO BRACE THE WALLS IS INSTALLED AND ADEQUATELY STRONG TO SUPPORT THE SOIL LOADING.

PLACE BACKFILL AS DIRECTED IN THE GEOTECHNICAL REPORT. IN THE ABSENCE OF A GEOTECHNICAL REPORT FOR THE PROJECT, PLACE BACKFILL IN LAYERS NOT EXCEEDING 6" IN DEPTH. MOISTEN EACH LAYER AND THOROUGHLY COMPACT THE SOIL PRIOR TO PLACING THE NEXT LAYER.

SOIL TYPE - 10' FILL COMPOSED OF CLAYEY SAND w/ GRAVEL (EXTERIOR ELEVATOR) - WEATHERED GREENSTONE BEDROCK INTERIOR ELEVATOR LOCATION).

CONCRETE

COMPLY WITH THE PROVISIONS OF CBC CHAPTER 19A AND ACI 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE AND COMMENTARY."

MATERIALS:

ASTM C150, TYPE II CEMENT: ASTM C33. NORMAL WEIGHT CONCRETE AGGREGATE: F'c = 3.000 PSI FOR ALL CONCRETE CONCRETE: SLUMP: 4" MAX SHRINKAGE: LESS THAN 0.05% w/c RATIO:

FLY ASH MEETING ASTM C618 CLASS F OR N MAY BE USED AS A PARTIAL REPLACEMENT FOR CEMENT. FLY ASH SHALL NOT EXCEED 25% [MEASURED BY WEIGHT] OF THE TOTAL CEMENTITIOUS CONTENT.

SUBMIT FOR REVIEW BY THE ENGINEER AND DSA THE PROPOSED MIX DESIGNS, IN CONFORMANCE WITH CBC SECTION 1905, REVIEWED AND APPROVED BY AN INDEPENDENT TESTING LABORATORY, CONCRETE MIX DESIGNS FOR EACH TYPE AND STRENGTH OF CONCRETE.

USE NO ADDITIVES OR ADMIXTURES UNLESS APPROVED BY THE ENGINEER

REINFORCE ALL CONCRETE UNLESS SPECIFICALLY MARKED "NOT REINFORCED." WHERE REINFORCEMENT IS NOT OTHERWISE INDICATED ON THE DRAWINGS, REINFORCE WITH THE FOLLOWING MINIMUM REQUIREMENTS:

REINFORCING EACH WAY #4 @ 12", CENTERED LESS THAN 8" 8" TO LESS THAN 10" #4 @ 9", CENTERED 10" TO LESS THAN 16" #4 @ 12", EACH FACE #5 @ 12", EACH FACE MORE THAN 16"

COORDINATE WITH THE TESTING AGENCY: DESIGNATED BY THE OWNER'S REPRESENTATIVE. FOR STRENGTH AND SLUMP TESTING AS SCHEDULED IN SPECIAL INSPECTION AND TESTING SCHEDULE. COST OF TESTING AS SCHEDULED WILL BE PAID BY THE OWNER. COST OF TESTING. REMOVAL AND REPAIR OF NONCONFORMING CONCRETE TO BE PAID BY THE CONTRACTOR.

COORDINATE WITH THE INSPECTION AGENCY DESIGNATED BY THE OWNER'S REPRESENTATIVE FOR INSPECTION OF REINFORCEMENT PLACEMENT AND PLACING OF CONCRETE AS SCHEDULED IN SPECIAL INSPECTION AND TESTING SCHEDULE.

NOTIFY THE OWNER'S REPRESENTATIVE AT LEAST ONE DAY IN ADVANCE

DESIGN FORMWORK IN ACCORDANCE WITH ACI 318 SECTION 6.1 AND ACI 347 "GUIDE TO FORMWORK FOR CONCRETE". USE BOND BREAKER OR FORM RELEASE AGENT FOR EASE OF REMOVAL FROM THE PLACED CONCRETE.

CHAMFER ALL CORNERS TO PREVENT DAMAGE.

OF CONCRETE PLACEMENT.

USE VIBRATORS TO CONSOLIDATE CONCRETE. DO NOT USE VIBRATORS TO MOVE CONCRETE. DO NOT VIBRATE FORMS OR USE FORM VIBRATORS.

FINISH FLATWORK TO STEEL TROWEL FINISH UNLESS INDICATED OTHERWISE ON THE DRAWINGS OR INSTRUCTED BY THE OWNER'S REPRESENTATIVE. ELEVATION OF FINISHED SLABS MAY VARY NO MORE THAN 1/8" IN 10'.

N. REMOVE FINS AND FILL VOIDS WITH APPROVED PATCHING MIX ON WALLS AND COLUMNS AND EXPOSED SURFACES.

THOROUGHLY SANDBLAST WITH COARSE SILICA SAND ALL CONSTRUCTION JOINTS TO CLEAN AND ROUGHEN THE ENTIRE JOINT, EXPOSING CLEAN COARSE AGGREGATE SOLIDLY EMBEDDED IN MORTAR MATRIX AND PAINT WITH A BONDING AGENT PRIOR TO PLACING NEW CONCRETE. COMPLY WITH THE PROVISIONS OF CBC SECTION 1906A.4 AND ACI 318, SECTION 11.7.9.

REPAIR STRUCTURAL AND FINISH DEFECTS AS DIRECTED BY THE OWNER'S REPRESENTATIVE.

- NON-SHRINK GROUT: MINIMUM COMPRESSIVE STRENGTH: f'c=3000 PSI AT 7 DAYS AND f'c=7000 PSI AT 28 DAYS.
- EPOXY GROUT: MINIMUM COMPRESSIVE STRENGTH: f'c=3000 PSI AT

7 DAYS AND f'c=7000 PSI AT 28 DAYS, U.O.N.

STRUCTURAL STEEL:

THREADED RODS:

COMPLY WITH THE PROVISIONS OF CBC CHAPTER 22A. AISC "SPECIFICATION

AS NOTED ON THE DRAWINGS

ASTM A193, GRADE B7

FOR STRUCTURAL STEEL BUILDINGS" AND AISC "CODE OF STANDARD PRACTICE". MATERIALS:

ASTM A992, GRADE 50 WIDE FLANGE MEMBERS: ASTM A36 OTHER ROLLED SHAPES: ASTM A36, A572 PLATES AND BARS: ASTM A53, TYPE S, GRADE B STRUCTURAL PIPE: ASTM A500, GRADE B HSS TUBING: AWS D1.1 WELDING: E70XX OR AS DETERMINED BY WELDING **ELECTRODES:** PROCEDURES ASTM A325N ASTM A307 ASTM A108 GRADES C1010 - C1020 WELDED STUDS: ASTM F1554, GRADE 36 ANCHOR BOLTS:

USE ASTM A325 NUT AND WASHER WHERE HSB ARE SPECIFIED.

HOLES MAY BE 1/16" LARGER THAN BOLT DIAMETER EXCEPT HOLES TO FIT OVER ANCHOR BOLTS MAY BE 1/4" DIAMETER LARGER THAN BOLT DIAMETER.

PAINT ONE SHOP COAT AND FIELD TOUCH UP WITH APPROVED PAINT: EXCLUDE AT HSB CONNECTION CONTACT SURFACES, WHERE FIREPROOFING IS TO BE INSTALLED AND AT LOCATIONS AND AREAS TO BE WELDED.

USE NON-SHRINK GROUT UNDER COLUMN BASES. GROUT TO COMPLY WITH CORPS OF ENGINEERS SPECIFICATION CRD-C621. FIVE STAR GROUT, MASTER BUILDERS, SIKA, OR EQUAL. MINIMUM COMPRESSIVE STRENGTH: f'c = 3000 PSI AT 7 DAYS AND f'c = 7000 PSI AT 28 DAYS.

USE COMPLETE JOINT PENETRATION WELDED JOINTS AT ALL SPLICES NOT INDICATED ON THE DRAWINGS.

SUBMIT QUALIFICATIONS AND CERTIFICATES FOR ALL WELDERS. SUBMIT WELDING PROCEDURES FOR APPROVAL BY THE ENGINEER. SUBMIT HEAT NUMBERS FOR ALL MEMBERS INCLUDED IN THE WORK.

ALL WELDS SPECIFIED ON DRAWINGS ARE NOT CLASSIFIED AS FIELD OR SHOP APPLIED. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE IF FIELD OR SHOP WELDING IS BEST SUITED FOR FABRICATION AND ERECTION OF SPECIFIC COMPONENTS.

FILLET WELD SIZES SHOWN ON THE DRAWINGS ARE MINIMUM SIZES, INCREASE WELD SIZE AS NECESSARY TO MEET AWS MINIMUM SIZES DUE TO BASE MATERIAL THICKNESS.

ALL GROOVE AND BUTT WELDS SHALL BE COMPLETE JOINT PENETRATION [CJP] TYPE, UON.

CARPENTRY:

COMPLY WITH THE PROVISIONS OF CBC CHAPTER 23A, NFPA "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" AND AITC "TIMBER CONSTRUCTION MANUAL."

LUMBER GRADING TO CONFORM TO WCLIB OR WWPA STANDARDS FOR THE SIZES SHOWN ON THE DRAWINGS. MOISTURE CONTENT NOT TO EXCEED 19% AT TIME OF INSTALLATION. USE PRESSURE TREATED LUMBER FOR MEMBERS EXPOSED TO WEATHER, SLEEPERS AND BEARING PLATES ON TOP OF ROOFS, AND AS NOTED ON ITEM D BELOW.

MATERIALS:

<u>FASTENER</u>

WALLS

USE THE FOLLOWING DOUGLAS FIR-LARCH GRADES WHERE GRADES ARE NOT

OTHERWISE INDICATED ON THE DRAWINGS: PS-1, STRUCT. 1, EXPOSURE

BLOCKING AND BRIDGING NAILERS

USE PRESERVATIVE PRESSURE TREATED DOUGLAS FIR-LARCH BEARING AN AWPA STAMP CONFORMING TO C2 OR C9 FOR WOOD OR PLYWOOD IN CONTACT WITH CONCRETE OR MASONRY.

ASTM F1667, COMMON WIRE TYPE [UON]

NAILING SCHEDULE - ALL MEMBERS THROUGHOUT BUILDING SHALL BE CONNECTED TOGETHER WITH FASTENERS LISTED IN, UNLESS A GREATER NUMBER ARE SHOWN OR CALLED FOR ELSEWHERE IN THE DRAWINGS. ALL NAILS SHALL BE COMMON WIRE NAILS, UON.

HOLE SIZE

PREDRILL HOLES FOR FASTENERS TO BE AS FOLLOWS:

3/4 DIA. IF NECESSARY TO PREVENT SPLITTING

INSTALL PLYWOOD WITH SHEETS CENTERED ACCURATELY OVER SUPPORTING MEMBERS. UNLESS OTHERWISE INDICATED ON THE DRAWINGS. LAY FACE GRAIN PERPENDICULAR TO SUPPORTS WITH END JOINTS STAGGERED. USE NO PIECES LESS THAN 24" x 24". USE 3x4 FLAT BLOCKING AT UNSUPPORTED EDGES WHERE INDICATED ON THE DRAWINGS AS BEING BLOCKED. WHERE FASTENING IS NOT OTHERWISE INDICATED ON THE DRAWINGS, FASTEN PLYWOOD WITH 10d COMMON WIRE NAILS PENETRATING THE FRAMING 1 5/8". SPACED AS FOLLOWS:

MIN. THICKNESS 9"o.c. 6"o.c.

12"o.c. ROOF MINIMUM WIDTH OF PLYWOOD SHEATHING: 24" AT ROOF AND; 12" AT WALLS. INSTALL STUD DEPTH BLOCKING, 6" NOM. MAX., AT ALL PLYWOOD EDGES IN SHEAR WALLS. USE BLOCKING 1" THICKER THAN SCHEDULED BLOCKING AT CONTINUOUS HORIZONTAL JOINTS. INSTALL BLOCKING AT ROOF.

FASTENERS FOR PRESSURE-PRESERVATIVE TREATED AND FIRE RETARDANT TREATED WOOD SHALL BE OF HOT-DIPPED ZINC COATED GALVANIZED, STAINLESS STEEL, SILICON BRONZE OR COPPER. FASTENERS REQUIRED TO BE CORROSION RESISTANT SHALL BE EITHER ZINC-COATED FASTENERS, ALUMINUM ALLOY WIRE FASTENERS OR STAINLESS STEEL FASTENERS. FASTENERS USED IN METAL FRAMING CONNECTORS SHALL BE OF MATERIAL OR FINISH COMPATIBLE WITH THE CONNECTOR.

4"o.c.

12"o.c.

STEEL REINFORCEMENT

COMPLY WITH THE PROVISIONS OF CBC SECTION 1907A AND CHAPTER 21A AND ACI 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE AND COMMENTARY."

MATERIALS

ASTM A615, GRADE 60 EXCEPT STIRRUPS AND TIES #3 AND SMALLER MAY BE GRADE 40 WELDED BARS: ASTM A706 ASTM A185

PLACE REINFORCEMENT CONTINUOUS WITH SPLICES STAGGERED. UNLESS OTHERWISE DETAILED, LAP BARS AS FOLLOWS: CONCRETE: ACI 318, CHAPTER 12

IN LIEU OF LAP SPLICE, SPLICING DEVICES CAPABLE OF DEVELOPING 125% OF THE YIELD CAPACITY OF THE BARS IN TENSION AND COMPRESSION CAN BE USED. SUBMIT SUBSTANTIATING DATA FOR REVIEW BY THE ENGINEER

HOLD REINFORCEMENT IN ITS TRUE POSITION WITH DEVICES SUFFICIENTLY NUMEROUS TO PREVENT DISPLACEMENT BY OPERATIONS BEFORE AND DURING CONCRETE PLACEMENT.

USE CBC STANDARD HOOKS, BENDS AND CLEARANCES BETWEEN BARS, UNLESS OTHERWISE DETAILED.

MINIMUM CONCRETE COVER AROUND REINFORCEMENT:

APPROVAL FROM ENGINEER.

CONCRETE PLACED AGAINST EARTH... FORMED, EXPOSED TO WEATHER OR EARTH. ..1-1/2" WALLS, BEAMS, COLUMN TIES OR SPIRALS ... INTERIOR SUSPENDED SLABS AND JOISTS. CONCRETE MASONRY FROM INSIDE SHELL.

SUBMIT HEAT NUMBERS FOR ALL REINFORCEMENT INCLUDED IN THE WORK. WHERE WELDING OF REINFORCEMENT IS DETAILED ON THE DRAWINGS,

SUBMIT QUALIFICATIONS AND CERTIFICATES FOR ALL WELDERS. SUBMIT WELDING PROCEDURES FOR APPROVAL BY THE ENGINEER AND DSA. FIELD BENDING OF REINFORCEMENT IS NOT PERMITTED WITHOUT PRIOR

ALL REBAR EPOXY DOWELS TO BE HILTI HIT-RE 500 ADHESIVE PER ESR-2322. ONLY WHERE SHOWN ON DRAWINGS AND APPROVED BY DSA

MECHANICAL ANCHORS

COMPLY WITH THE PROVISIONS OF CBC SECTION 1911A, 1912A AND 1916A.8.

EXPANSION ANCHORS SHALL BE CARBON STEEL HILTI KWIK BOLT TZ [ICC ESR-1917] OR SIMPSON STRONG-BOLT [ICC ESR-1771].

ANCHOR SHALL BE INSTALLED IN ACCORDANCE WITH THE REQUIREMENTS GIVEN IN THE ICC RESEARCH COMMITTEE RECOMMENDATIONS FOR THE SPECIFIC ANCHOR.

THE CONTRACTOR SHALL COORDINATE THE CONSTRUCTION SUCH THAT INTERFERENCE OF REINFORCING STEEL WITH CONCRETE ANCHOR PLACEMENT DOES NOT OCCUR. IF REINFORCING STEEL IS ENCOUNTERED DURING DRILLING, ADJUST THE ANCHOR LOCATION IF POSSIBLE AND NOTIFY THE OWNER'S REPRESENTATIVE. ABANDONED HOLES SHALL BE FILLED WITH NON-SHRINK GROUT. ANCHORS SHALL BE SET WITHIN 3 INCHES OF THEIR SPECIFIED LOCATION, BUT AT LEAST 1 INCH FROM ANY ABANDONED HOLE. CARE SHALL BE TAKEN NOT TO BREAK OR DAMAGE REINFORCING STEEL DURING DRILLING, UNLESS OTHERWISE DIRECTED BY THE OWNER'S REPRESENTATIVE.

TESTS SHALL BE IN THE PRESENCE OF THE OWNER'S PROJECT INSPECTOR OR TESTING LABORATORY AND THE TEST RESULTS SUBMITTED TO THE PROJECT ENGINEER. TEST REQUIREMENTS FOR EXPANSION ANCHORS USED IN METAL SUSPENSION SYSTEM FOR LAY IN PANEL CEILINGS SHALL BE IN ACCORDANCE WITH THE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS. TEST VALUES LISTED ARE FOR A TORQUE WRENCH TEST. THE APPLICABLE TEST TORQUE MUST BE REACHED WITHIN ONE-HALF TURN OF THE NUT.

IN THE EVENT OF ANY TEST FAILURE, TEST ALL ANCHORS OF THE SAME CATEGORY NOT PREVIOUSLY TESTED UNTIL TWENTY CONSECUTIVE PASS. THEN RESUME INITIAL TESTING FREQUENCY. PATCH ALL HOLES WHERE FASTENERS ARE REMOVED USING NON-SHRINK GROUT, PACKED SOLIDLY.

ALLOWABLE LOADS LISTED ARE 100% OF THE ICC REPORT VALUES WITH SPECIAL INSPECTION. REDUCE VALUES AS SPECIFIED BY THE APPLICABLE ICC REPORT WHEN INSTALLED WITH EDGE DISTANCES OR ANCHOR SPACING LESS THAN DIMENSIONS NOTED.

TABULATED ALLOWABLE TENSION VALUES ARE FOR INSTALLATION WITH SPECIAL

ANCHORS INSTALLED IN NORMAL WEIGHT CONCRETE, MINIMUM CONCRETE STRENGTH IS 3000 PSI. ANCHORS ARE MADE OF CARBON STEEL FOR

INSIDE INSTALLATIONS.

BOLT DIAMETER	MIN. EMBEDMENT UON	ALLOWABLE SHEAR	ALLOWABLE TENSION	EDGE DISTANCE	SPACING	MIN. MEMBER THICKNESS	TORQUE TEST
(INCHES)	(INCHES)	(POUNDS)	(POUNDS)	(INCHES)	(INCHES)	(INCHES)	(FT-LBS
HILTI KWIF	K BOLT TZ,	STATIC (ASD): SEE NOT	E 1			
3	2	1669	1155	48	3 5	4	25
1/2	2 34	2974	1223 2500	5½ 7½	48 32	4	40
5	38 4	4901	2388 3458	6½ 8¾	43 44	5	60
3 4	3 ³ / ₄	7396	3139 4475	10	9½ 7¾	6	110
			44/3	<u> </u>	/4	8.9	
HILTI KWIK	K BOLT TZ, :	SEISMIC/WIN	D (ASD): S	EE NOTE 1			
CO	2	999	1102	43	3 §	4	25
1 2	2 31/4	2839	1167 2386	5½ 7½	4 1 3 <u>1</u>	4 6	40
5300	31 4	4678	2280 3301	61 62 83	4 3 4 4	5	60
3 4	33 43 43	6313	2997 4272	10 9	9½ 7¾ 7¾	6	110
NOTE 1:	USE SPAC		DISTANCE AD.	JUSTMENT C	HART (FIG.	4 PER ES	R-1917]
	23	1570	1235	9	4	4½	50
2	37 38	3045	1290	7 7	4	6	
5380	38 58	4245 4865	1670 2320	11 9 5	6 ‡ 6 ‡	5½ 7%	85
3)4	4 8 5 3	5775 8275	2270 4090	13½ 11¾	6‡ 6‡	6 4 84	180
SIMPSON	STRONG BOL	T, SEISMIC/	WIND (ASD):		9 B		
1 2	23 37 38	1500 2905	1180 1230	9 7 <mark>7</mark>	4	4½ 6	50
5300	3 <u>3</u> 5 <u>1</u>	3715 4305	1590 2215	11 95	6 1	5½ 78	85
<u>3</u>	4 1 5 3	5510 7080	2165 3905	13½ 11¾	6 1	64 84	180
	V4	7,000		1 14	~ *		
	THE ACADES AND THE SAME AND ASSESSMENT OF THE SAME ASSESSM	L					

ANCHOR DIAMETER REFERS TO THE THREAD SIZE FOR THE WEDGE AND TO THE ANCHOR OUTSIDE DIAMETER FOR THE SLEEVE CATEGORY.

APPLY PROOF TEST LOADS TO WEDGE AND SLEEVE ANCHORS WITHOUT REMOVING THE NUT IF POSSIBLE. IF NOT. REMOVE NUT AND INSTALL A THREADED COUPLER TO THE

FOR SLEEVE INTERNALLY THREADED CATEGORIES, VERIFY THAT THE ANCHOR IS NOT PREVENTED FROM WITHDRAWING BY A BASE PLATE OR OTHER FIXTURES. IF RESTRAINT IS FOUND, LOOSEN AND SHIM OR REMOVE FIXTURE(S) PRIOR TO TESTING.

TEST EQUIPMENT IS TO BE CALIBRATED BY AN APPROVED TESTING LABORATORY IN ACCORDANCE

TESTING SHOULD OCCUR 24 HOURS MINIMUM AFTER INSTALLATION OF THE SUBJECT ANCHORS.

SAME TIGHTNESS OF THE ORIGINAL NUT USING A TORQUE WRENCH AND APPLY LOAD.

REACTION LOADS FROM TEST FIXTURES MAY BE APPLIED CLOSE TO THE ANCHOR BEING TESTED. PROVIDED THE ANCHOR IS NOT RESTRAINED FROM WITHDRAWING BY THE FIXTURE(S).

WITH STANDARD RECOGNIZED PROCEDURES.

ALL EXPANSION ANCHORS OF EACH SIZE SHALL BE TENSION TESTED. WHERE ANCHORS ARE USED FOR EQUIPMENT ANCHORAGE, 50% OF ALTERNATE BOLTS IN A GROUP SHALL BE TENSION

DRAWING INDEX

GENERAL NOTES FIRST FLOOR PLAN

SECOND FLOOR PLAN

THIRD FLOOR PLAN ROOF AND HIGH ROOF PLANS

INTERIOR ELEVATOR PARTIAL FRAMING PLANS

EXTERIOR ELEVATOR FOUNDATION PLAN

SD2.6 FOUNDATION/STAIR DEMOLITION PLAN EXTERIOR ELEVATOR FRAMING PLANS

SECTION

SECTION

CONCRETE DETAILS CONCRETE DETAILS

STEEL DETAILS LIGHT GAGE STEEL TYPICAL DETAILS

LIGHT GAGE STEEL DETAILS

architecture planning

interiors

Bunton Clifford Associates, Inc. 210 Hammond Ave Fremont, California 94539 [T] 510.445.1000 [F] 510.445.1005 www.BCAincOnline.com

ARCHITECT	ENGINEER			
* Ren.: 09/30/09 *	No. \$2358 Exp. 3-31-10			
This sheet is part of a set and is	s not to be used alone.			

2. This sheet is not to be used for construction unless the architect's

stamp and signature appear on the drawings and the status box

indicates drawings have been released for construction. 3. These plans and prints thereof, as instruments of service. are owned by the architect and are for use on this project only. Reproduction and/or distribution without the prior written consent of the architect is forbidden. 4. Copyright Bunton Clifford Associates, 2007

DA	1
DSA PLAN CHECK	
DSA BACK CHECK	_
BIDDING (BID #86593)	_
CONSTRUCTION	_
FILE NO. 41-C1	-
identification stamp div. of the state architect	
ACFLS SS PART DATE_MAR 1 9 2009	

BUILDINGS 5 & 6 RENOVATIONS

San Mateo County Community College District

DSA BACK-CHECK

CAÑADA COLLEGE 4200 Farm Hill Boulevard

Redwood City, CA 94061

GENERAL NOTES

Drawing Number

AS NOTED Project Number

07013