ABBREVIATIONS

A.B.	ANCHOR BOLT
A.C.	ASPHALT CONCRETE
ALT.	ALTERNATE
ALUM.	ALUMINUM
APPROX.	APPROXIMATE [LY]
ARCH.	ARCHITECT [URAL]
BETW.	BETWEEN
BLDG.	BUILDING
BLK.	BLOCK [ING]
BM.	BEAM
B.O.F.	BOTTOM OF FOOTING
B.O.P	BOTTOM OF PIER CAP
BOT.	BOTTOM
BSMT.	BASEMENT
C.C.	CENTER TO CENTER
C.I.A.	COVERT INJECTION ADHESIVE
C.J.	CONSTRUCTION JOINT. COLD JOINT.
CLG.	CEILING
CLR.	CLEAR
C.M.U.	CONCRETE MASONRY UNIT
COL.	COLUMN
CONC.	CONCRETE
CONN.	CONNECTION
CONSTR.	CONSTRUCTION
CONT.	CONTINUOUS
CONTR.	CONTRACTOR
CSK.	COUNTERSINK
CTBR.	COUNTERBORE
℄	CENTER LINE
DBL.	DOUBLE
DET.	DETAIL
D.F.	DOUGLAS FIR
DIAG.	DIAGONAL
DIA. OR ø	DIAMETER
DIM.	DIMENSION
DN.	DOWN
DO.	DITTO
DWG.	DRAWING
E	EAST
EA.	EACH
E.B.	EXPANSION BOLT
E.D.	EDGE OF DECK
E.E.	EACH END
E.F.	EACH FACE
E.J.	EXPANSION JOINT
EL.	ELEVATION DENOTING HEIGHT. [EL. --1'-0"]
ELEC.	ELECTRICAL
ELEV.	ELEVATION. ELEVATOR
ENG.	ENGINEER [ED]
EQ.	EQUAL
E.S.	EACH SIDE
EST.	ESTIMATE [D]
E.W.	EACH WAY
E.W.E.F.	EACH WAY EACH FACE
EXIST. [E]	EXISTING
F.B.	FLAT BAR
FDN.	FOUNDATION
FIN.	FINISH [ED]
FL.	FLOOR
FLG.	FLANGE
F.O.C.	FACE OF CONCRETE
F.O.M.	FACE OF MASONRY
F.O.S.	FACE OF STUDS
F.S.	FAR SIDE
FTG.	FOOTING
[F]	FUTURE
GA.	GAUGE
GALV.	GALVANIZED
G.I.	GALVANIZED IRON
G.L.B.	GLULAM BEAM
GR.	GRADE
HDR.	HEADER
HEX.	HEXAGONAL
HGR.	HANGER
HORIZ. [H]	HORIZONTAL [#4H @ 12"]
H.P.	HIGH POINT
H.S.B.	HIGH STRENGTH BOLT
HT.	HEIGHT
I.D.	INSIDE DIAMETER
INCL.	INCLUDING
INSUL.	INSULATION
INV.	INVERT
J.	JOIST
J.H.	JOIST HANGER
JT.	JOINT
L	LENGTH
L.H.	LEFT HAND
L.L.	LIVE LOAD
LW.	LIGHTWEIGHT
MAT.	MATERIAL
MAX.	MAXIMUM
M.B.	MACHINE BOLT
MECH.	MECHANICAL
MET.	METAL
MFR.	MANUFACTURER
M.I.	MALLEABLE IRON
MIN.	MINIMUM
MISC.	MISCELLANEOUS
MULL.	MULLION
[N]	NEW
N	NORTH
N.I.C.	NOT IN CONTRACT
NO.	NUMBER [#6]
NOM.	NOMINAL
N.S.	NEAR SIDE
N.T.S.	NOT TO SCALE
O.C.	ON CENTER
O.D.	OUTSIDE DIAMETER
O.F.C.	OUTSIDE FACE OF CONCRETE
O.F.S.	OUTSIDE FACE OF STUDS
O.H.	OPPOSITE HAND
OPNG.	OPENING
OPP.	OPPOSITE
O. TO O.	OUT TO OUT
O.W.J.	OPEN WEB JOIST
PC.	PIECE
P.C.F.	POUNDS PER CUBIC FOOT
P.D.F.	POWDER DRIVEN FASTENER
P.J.	POUR JOINT
PL. OR ℙ	PLATE
P.L.F.	POUNDS PER LINEAL FOOT
PLY.	PLYWOOD
P.S.F.	POUNDS PER SQUARE FOOT
P.S.I.	POUNDS PER SQUARE INCH
PT.	POINT
R	RISERS
RAD. [R]	RADIUS
R.D.	ROOF DRAIN
RD. OR	ROUND
REINF.	REINFORCED [ING]
REQD.	REQUIRED
REV.	REVISION. REVISED
R.H.	RIGHT HAND
RM.	ROOM
RO.	ROUGH
R.O.	ROUND OPENING
S	SOUTH
SCHEDL.	SCHEDULE
SECT.	SECTION
SH.	SHEET
SIM.	SIMILAR
S.M.	SHEET METAL
S.O.G.	SLAB ON GRADE
SPECS.	SPECIFICATIONS
SQ. OR ◻	SQUARE
STAG.	STAGGERED
STD.	STANDARD
STL.	STEEL
STRUCT.	STRUCTURAL
SYM.	SYMMETRICAL
THK.	THICK
T.O.F.	TOP OF FOOTING
T.O.P.	TOP OF PLYWOOD
T.O.S.	TOP OF STEEL/SLAB
TR.	THREADS
TYP.	TYPICAL
U.O.N.	UNLESS OTHERWISE NOTED
VERT. [V]	VERTICAL [#4V @ 12"]
V.I.F.	VERIFY IN FIELD
W	WEST, WIDTH
W/	WITH
W.P.	WORK POINT OR WATERPROOF
WT.	WEIGHT
W.W.F.	WELDED WIRE FABRIC
YD.	YARD

DRAWING INDEX

S0.1	GENERAL NOTES
S2.1	FOUNDATION AND GROUND AND FIRST FLOOR PLAN
S2.2	SECOND FLOOR AND ROOF PLAN
S5.1	CONCRETE DETAILS
S5.2	CONCRETE DETAILS
S5.3	CONCRETE DETAILS
S5.4	CONCRETE DETAILS
S5.5	CONCRETE DETAILS
S7.2	TYPICAL STEEL DETAILS
S7.3	STEEL DETAILS
S9.1	ELEVATOR TOWER PLANS AND DETAILS
S9.2	ELEVATOR TOWER DETAILS

LEGEND:

3.5A

SECTION NUMBER

SHEET ON WHICH SECTION IS SHOWN

POURED IN PLACE CONCRETE

EXISTING CONCRETE

BRACE UP

BRACE DOWN

GENERAL NOTES

GENERAL:

A. DESIGN CODE: CALIFORNIA BUILDING CODE [CBC] TITLE 24, PART 2; 1998 EDITION

B. SEISMIC DESIGN CRITERIA:
SEISMIC ZONE: 4
IMPORTANCE FACTOR: 1.15
R 4.5
SOIL TYPE: SC
Na=1.5, Nv=2.0
REDUNDANCY FACTOR, ρ = 1.0

C. WIND DESIGN CRITERIA:
BASIC WIND SPEED: 70 MPH
EXPOSURE: C
IMPORTANCE FACTOR: 1.0

D. PERFORM THE WORK IN THE ORDER INDICATED ON THE DRAWINGS WHERE WORK REQUIRES SEQUENTIAL OPERATIONS.

E. WHEN REQUESTING SUBSTITUTIONS FOR PRODUCTS, PROCEDURES, METHODS OR MATERIALS SPECIFIED FOR THE PROJECT, SUBMIT ENGINEERING DATA ESTABLISHING EQUIVALENCE AND ICBO ACCEPTANCE NUMBER, IF APPLICABLE, FOR REVIEW BY THE ARCHITECT AND APPROVAL BY DSA PRIOR TO INCORPORATING INTO THE WORK.

F. WHERE STANDARDS ARE LISTED, USE THE LATEST DSA ACCEPTED AND APPROVED ISSUE.

DIMENSIONS AND DATUM:

A. DIMENSIONS ARE GIVEN TO CENTERLINE OF COLUMNS AND BEAMS OR FACE OF WALLS AND ROUGH CONCRETE SURFACES, UNLESS OTHERWISE NOTED.

B. ELEVATIONS ARE GIVEN WITH REFERENCE TO EXISTING FINISHED GROUND FLOOR ELEVATION: EL. = ±0'-0" [DATUM ELEVATION = 609'-0"]

EXISTING CONSTRUCTION:

A. DETAILS AND DIMENSIONS FOR EXISTING CONSTRUCTION HAVE BEEN TAKEN FROM DRAWINGS. VERIFY THAT THE EXISTING CONDITIONS AFFECTED BY THE WORK ARE AS INDICATED ON THE DRAWINGS AND, IN CASE OF DISCREPANCY, PROCEED ONLY AFTER RECEIVING INSTRUCTIONS FROM THE ARCHITECT.

B. VERIFY THAT DISCONNECTING, REMOVING OR DEMOLISHING ANY EXISTING CONSTRUCTION WILL NOT CAUSE INSTABILITY IN ADJACENT CONSTRUCTION PRIOR TO COMMENCING WORK.

FOUNDATION:

A. COMPLY WITH THE PROVISIONS OF CHAPTER 18A CBC.

B. WHERE PRACTICABLE, MAKE EXCAVATIONS AS NEAR AS POSSIBLE TO THE NEAT LINES REQUIRED BY THE SIZE AND SHAPE OF THE STRUCTURE. EXCAVATE NO MATERIAL UNNECESSARILY.

C. WHERE EXCAVATIONS CANNOT BE MAINTAINED FOR A NEAT POUR, FORM THE SIDES. WHEN POURING FOOTINGS NEAT, ADD ONE INCH EACH SIDE OF FOOTINGS TO THE SIZES SHOWN ON THE DRAWINGS.

D. ELEVATIONS OF BOTTOMS OF FOOTINGS HAVE BEEN ESTABLISHED TO REACH COMPETENT NATURAL SOILS, OR ENGINEERED FILL AS DETERMINED FROM GEOTECHNICAL REPORT BY LOWNEY ASSOCIATES, DATED AUG. 15, 2000 AND SUPPLEMENTARY LETTER DATED JAN. 23, 2002. THIS MATERIAL IS CAPABLE OF SUPPORTING LOADS AS INDICATED BELOW:

DEAD LOAD	6,000 PSF
DEAD LOAD + LIVE LOAD	10,000 PSF
TOTAL LOAD INCL. SEISMIC	13,500 PSF

E. AS EXCAVATION PROGRESSES, CONDITIONS MAY DEVELOP REQUIRING CHANGES IN ELEVATIONS OF FOOTINGS SHOWN ON THE DRAWINGS. MAKE SUCH CHANGES ONLY AS DIRECTED BY THE ARCHITECT.

F. WHERE BACKFILL IS TO BE PLACED AGAINST WALLS BEFORE THEY HAVE ATTAINED THEIR DESIGN STRENGTH, SHORE THE WALLS TO SUPPORT THE SOIL LOADING. THE SHORING IS TO REMAIN IN PLACE UNTIL THE WALL HAS ATTAINED ITS DESIGN STRENGTH AND/OR OTHER CONSTRUCTION INTENDED TO BRACE THE WALLS IS INSTALLED AND ADEQUATELY STRONG TO SUPPORT THE SOIL LOADING.

G. PLACE BACKFILL AS DIRECTED IN THE GEOTECHNICAL REPORT. MOISTEN EACH LAYER AND THOROUGHLY COMPACT THE SOIL PRIOR TO PLACING THE NEXT LAYER.

H. SOIL TYPE: SANDY CLAY

CONCRETE:

A. COMPLY WITH THE PROVISIONS OF CHAPTER 19A CBC AND ACI 318-95 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE".

B. MATERIALS:

CEMENT:	A.S.T.M. C150, TYPE II
AGGREGATE:	GRANITE OR LIMESTONE ONLY, ASTM C33, UON.
CONCRETE:	F'c=4,000 PSI FOR ALL CONCRETE
SLUMP:	3.5" MAX.
SHRINKAGE:	LESS THAN 0.05%.

C. SUBMIT FOR REVIEW BY THE ARCHITECT AND APPROVAL BY DSA, THE PROPOSED MIX DESIGNS, REVIEWED AND APPROVED BY AN INDEPENDENT TESTING LABORATORY, CONCRETE MIX DESIGNS FOR EACH TYPE AND STRENGTH OF CONCRETE, INCLUDING SHRINKAGE HISTORY.

D. USE NO ADDITIVES OR ADMIXTURES UNLESS APPROVED BY THE ARCHITECT AND DSA. USE NO OTHER CEMENTITIOUS MATERIALS.

E. REINFORCE ALL CONCRETE UNLESS SPECIFICALLY MARKED "NOT REINFORCED". WHERE REINFORCEMENT IS NOT OTHERWISE INDICATED ON THE DRAWINGS, REINFORCE WALLS WITH THE FOLLOWING MINIMUM REINFORCEMENT:

WALL THICKNESS	REINFORCING EACH WAY
9"	#4 @ 12", EACH FACE

F. PLACE CONCRETE IN WALLS, COLUMNS AND BEAMS IN CONTINUOUS POURS UNLESS APPROVED BY THE ARCHITECT IN ADVANCE.

LIGHT-WEIGHT CONCRETE:

A. MATERIALS:

CEMENT:	A.S.T.M. C150, TYPE II
AGGREGATE:	A.S.T.M. C330
CONCRETE:	F'c=3,000 PSI

SPRAYED ON CONCRETE [GUNITE]:

A. COMPLY WITH THE PROVISIONS OF SECTION 1924A CBC AND ACI 506.2 LATEST EDITION. "SPECIFICATION FOR MATERIALS, PROPORTIONING AND APPLICATION OF SHOTCRETE".

B. BE FAMILIAR WITH AND COMPLY WITH THE PRACTICES CONTAINED IN ACI 506 "GUIDE TO SHOTCRETE".

C. MATERIALS:

CEMENT:	A.S.T.M. C-150, TYPE II
F'c=	4000 PSI AT 28 DAYS

D. SUBMIT FOR REVIEW BY THE ARCHITECT AND DSA, REVIEWED AND APPROVED BY AN INDEPENDENT TESTING LABORATORY, THE PROPOSED SHOTCRETE MIX DESIGNS. APPLY NO SHOTCRETE TO THE WORK UNTIL PRECONSTRUCTION TEST CORES INDICATE CONFORMANCE TO THE STRENGTH REQUIREMENTS.

E. NOTIFY THE ARCHITECT AND DSA AT LEAST TWO [2] DAYS PRIOR TO SHOTCRETE APPLICATION, INCLUDING PRECONSTRUCTION TEST PANELS, APPLICATION TO THE WORK AND QUALITY ASSURANCE TEST PANELS.

F. THOROUGHLY CLEAN BY SANDBLASTING ALL SURFACES TO RECEIVE SHOTCRETE.

STEEL REINFORCEMENT:

A. COMPLY WITH THE PROVISIONS OF CHAPTERS 19A & 21A CBC AND ACI 318-95 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE".

B. MATERIALS:

BARS:	A.S.T.M. A-615, GRADE 60 EXCEPT STIRRUPS AND TIES
WELDED BARS:	A.S.T.M. A-706
WWF:	A.S.T.M. A-185.

C. PLACE REINFORCEMENT CONTINUOUS WITH SPLICES STAGGERED. UNLESS OTHERWISE DETAILED, LAP BARS AS FOLLOWS:

CONCRETE: CBC SECTION 1912A AND 5.1E/S5.1.

D. HOLD REINFORCEMENT IN ITS TRUE POSITION WITH DEVICES SUFFICIENTLY NUMEROUS TO PREVENT DISPLACEMENT BY OPERATIONS BEFORE AND DURING CONCRETE PLACEMENT.

E. USE UBC STANDARD HOOKS, BENDS AND CLEARANCES BETWEEN BARS, UNLESS OTHERWISE DETAILED. SEE OTHER REQUIREMENTS UNDER SHOTCRETE.

F. MINIMUM CONCRETE COVER AROUND REINFORCEMENT:
CONCRETE PLACED AGAINST EARTH.....3"
FORMED, EXPOSED TO WEATHER OR EARTH.....2"
WALLS, BEAMS, COLUMN TIES OR SPIRALS.....$1\frac{1}{2}$"
INTERIOR SUSPENDED SLABS AND JOISTS.....$\frac{3}{4}$"

G. WHERE INDICATED ON PLAN USE MECHANICAL COUPLERS BY LENTON, OR APPROVED EQUAL. COUPLERS SHALL BE STAGGERED 24" MINIMUM FOR ADJACENT BARS. ALL CORED HOLES SHALL BE FILLED WITH NON-SHRINK GROUT (U.O.N.).

H. CONTRACTORS SHALL FIELD VERIFY LOCATIONS OF EXISTING REBARS PRIOR TO CORE DRILLING.

CONSTRUCTION JOINTS:

A. FOR CONSTRUCTION JOINTS IN GENERAL, SEE SPECIFICATIONS AND CBC SECTION 1906A.4.

B. THOROUGHLY SANDBLAST WITH COARSE SILICA SAND ALL CONSTRUCTION JOINTS TO CLEAN AND ROUGHEN THE ENTIRE SURFACE OF THE JOINT, EXPOSING CLEAN COARSE AGGREGATE SOLIDLY EMBEDDED IN MORTAR MATRIX.

EXPANSION ANCHORS

A. EXPANSION ANCHORS SHALL BE CARBON STEEL HILTI KWIK BOLT II (ICBO REPORT NO. 4627) OR ITW RAMSET/RED HEAD TRUBOLT WEDGE ANCHORS. (ICBO REPORT NO. 1372)

B. ANCHOR SHALL BE INSTALLED IN ACCORDANCE WITH THE REQUIREMENTS GIVEN IN THE ICBO RESEARCH COMMITTEE RECOMMENDATIONS FOR THE SPECIFIC ANCHOR.

C. THE CONTRACTOR SHALL COORDINATE THE CONSTRUCTION SUCH THAT INTERFERENCE OF REINFORCING STEEL WITH CONCRETE ANCHOR PLACEMENT DOES NOT OCCUR. IF REINFORCING STEEL IS ENCOUNTERED DURING DRILLING, ADJUST THE ANCHOR LOCATION IF POSSIBLE AND NOTIFY THE OWNER'S REPRESENTATIVE. ABANDONED HOLES SHALL BE FILLED WITH GROUT. ANCHORS SHALL BE SET WITHIN 3 INCHES OF THEIR SPECIFIED LOCATION, BUT AT LEAST 1 INCH FROM ANY ABANDONED HOLE. CARE SHALL BE TAKEN NOT TO BREAK OR DAMAGE REINFORCING STEEL DURING DRILLING, UNLESS OTHERWISE DIRECTED BY THE OWNER'S REPRESENTATIVE.

D. TESTS SHALL BE IN THE PRESENCE OF THE OWNER'S PROJECT INSPECTOR OR TESTING LABORATORY AND THE TEST RESULTS SUBMITTED TO THE PROJECT ENGINEER. TEST REQUIREMENTS FOR EXPANSION ANCHORS USED IN METAL SUSPENSION SYSTEM FOR LAY IN PANEL CEILINGS SHALL BE IN ACCORDANCE WITH THE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS. TEST VALUES LISTED ARE FOR A TORQUE WRENCH TEST. THE APPLICABLE TEST TORQUE MUST BE REACHED WITHIN ONE-HALF TURN OF THE NUT.

E. IN THE EVENT OF ANY TEST FAILURE, TEST ALL ANCHORS OF THE SAME CATEGORY NOT PREVIOUSLY TESTED UNTIL TWENTY CONSECUTIVE PASS. THEN RESUME INITIAL TESTING FREQUENCY. PATCH ALL HOLES WHERE FASTENERS ARE REMOVED USING GROUT, PACKED SOLIDLY.

F. ALLOWABLE LOADS LISTED ARE 80% OF THE ICBO REPORTED VALUES. REDUCE VALUES AS SPECIFIED BY THE APPLICABLE ICBO REPORT WHEN INSTALLED WITH EDGE DISTANCES OR ANCHOR SPACING LESS THAN DIMENSIONS NOTED.

TENSION TEST VALUES FOR EACH ANCHOR TYPE ARE EQUAL TO 2 TIMES THE ALLOWABLE TENSION VALUES LISTED.

ANCHORS INSTALLED IN REGULAR CONCRETE, MINIMUM CONCRETE STRENGTH IS 3000 PSI.

BOLT DIAMETER (INCHES)	MIN. EMBEDMENT U.N.O. (INCHES)	ALLOWABLE SHEAR (POUNDS)	ALLOWABLE TENSION (POUNDS)	EDGE DISTANCE (INCHES)	SPACING (INCHES)	TORQUE TEST (FT-LBS)
HILTI KWIK BOLT II:						
$\frac{1}{4}$	2	320	420	$3\frac{5}{8}$	4	7
$\frac{3}{8}$	3	880	968	5	5	25
$\frac{1}{2}$	4	1472	1600	$6\frac{3}{4}$	7	45
$\frac{5}{8}$	5	2500	2136	$8\frac{1}{4}$	8	95
$\frac{3}{4}$	6	3380	2900	$9\frac{3}{4}$	$9\frac{1}{2}$	225
ITW RAMSET/RED HEAD TRUBOLT:						
$\frac{1}{4}$	2	336	540	2	4	8
$\frac{3}{8}$	3	814	942	$3\frac{3}{4}$	6	25
$\frac{1}{2}$	$4\frac{1}{8}$	1448	1430	$5\frac{1}{4}$	$6\frac{1}{4}$	55
$\frac{5}{8}$	$5\frac{1}{8}$	2150	2150	$6\frac{7}{16}$	$7\frac{3}{4}$	90
$\frac{3}{4}$	$6\frac{5}{8}$	4406	2868	$8\frac{5}{16}$	10	175

G. ANCHOR DIAMETER REFERS TO THE THREAD SIZE FOR THE WEDGE AND TO THE ANCHOR OUTSIDE DIAMETER FOR THE SLEEVE CATEGORY.

H. APPLY PROOF TEST LOADS TO WEDGE & SLEEVE ANCHORS WITHOUT REMOVING THE NUT IF POSSIBLE. IF NOT, REMOVE NUT & INSTALL A THREADED COUPLER TO THE SAME TIGHTNESS OF THE ORIGINAL NUT USING A TORQUE WRENCH & APPLY LOAD.

I. FOR SLEEVE INTERNALLY THREADED CATEGORIES, VERIFY THAT THE ANCHOR IS NOT PREVENTED FROM WITHDRAWING BY A BASE PLATE OR OTHER FIXTURES. IF RESTRAINT IS FOUND, LOOSEN AND SHIM OR REMOVE FIXTURE(S) PRIOR TO TESTING.

J. REACTION LOADS FROM TEST FIXTURES MAY BE APPLIED CLOSE TO THE ANCHOR BEING TESTED, PROVIDED THE ANCHOR IS NOT RESTRAINED FROM WITHDRAWING BY THE FIXTURE(S).

K. TEST EQUIPMENT IS TO BE CALIBRATED BY AN APPROVED TESTING LABORATORY IN ACCORDANCE WITH STANDARD RECOGNIZED PROCEDURES.

L. TESTING SHOULD OCCUR 24 HOURS MINIMUM AFTER INSTALLATION OF THE SUBJECT ANCHORS.

M. ALL EXPANSION ANCHORS SHALL BE TENSION TESTED. WHERE ANCHORS ARE USED FOR EQUIPMENT ANCHORAGE, 50% OR ALTERNATE BOLTS IN A GROUP SHALL BE TENSION TESTED.

STRUCTURAL STEEL:

A. COMPLY WITH THE PROVISIONS OF CHAPTER 22A CBC, AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND AISC "CODE OF STANDARD PRACTICE".

B. MATERIALS:

ROLLED SHAPES:	A.S.T.M. A-36
PLATES AND BARS:	A.S.T.M. A-36
SEAMLESS PIPE:	A.S.T.M. A-53, TYPE S, GRADE B
SEAMLESS TUBING:	A.S.T.M. A-500, GRADE B
WELDING:	AWS D1.1
ELECTRODES:	E70XX OR AS DETERMINED BY WELDING PROCEDURES.
H.S.B.:	A.S.T.M. A-325X △1
M.B.:	A.S.T.M. A-307
WELDED STUDS:	A.S.T.M. A-108-58-T, C-1015 COLD ROLLED.
ANCHOR BOLTS:	A.S.T.M. A-307 AS NOTED ON THE DRAWINGS.

THREADED THRU BOLTS SHALL BE A572-GR.50.
EXPANSION BOLTS ARE HILTI KWIK BOLT II OR EQUAL.

C. USE COMPLETE PENETRATION JOINTS AT ALL SPLICES NOT INDICATED ON THE DRAWINGS.

D. PAINT ONE SHOP COAT AND FIELD TOUCH UP WITH APPROVED PAINT: EXCLUDE AT H.S.B. CONNECTION CONTACT SURFACES, WHERE FIREPROOFING IS TO BE INSTALLED AND AT LOCATIONS AND AREAS TO BE WELDED.

E. HOLES MAY BE $\frac{1}{16}$" LARGER THAN BOLT DIAMETER EXCEPT HOLES TO FIT OVER ANCHOR BOLTS MAY BE $\frac{1}{4}$" DIAMETER LARGER THAN BOLT DIAMETER.

F. SUBMIT QUALIFICATIONS AND CERTIFICATES FOR ALL WELDERS. SUBMIT WELDING PROCEDURES FOR APPROVAL BY THE ARCHITECT AND DSA. SUBMIT HEAT NUMBERS FOR ALL MEMBERS INCLUDED IN THE WORK.

METAL DECKING:

A. COMPLY WITH THE PROVISIONS OF CHAPTER 22A, CBC AND SDI STANDARDS.

B. MATERIALS:

DECKING:	A.S.T.M. A-446, GRADE B OR A.S.T.M. A-611, GRADE C.
GALVANIZING:	A.S.T.M. A-525, G-60.

C. METAL DECKING DESIGNATIONS SHOWN ON THE DRAWINGS REFER TO VERCO PRODUCTS UNLESS OTHERWISE NOTED.

D. PROVIDE SHEETS IN THE LONGEST LENGTH CONSISTENT WITH THE REQUIREMENTS OF THE WORK. USE NO SHEETS LESS THAN 18" WIDE.

E. LAYOUT ALL OPENINGS PRIOR TO CUTTING. WHERE THE OPENINGS WOULD RESULT IN LOSS OF SUPPORT FOR THE DECKING, NOTIFY THE ARCHITECT AND PROCEED ONLY AFTER RECEIVING INSTRUCTIONS.

F. USE $\frac{3}{8}$" MINIMUM EFFECTIVE DIAMETER PUDDLE WELDS WHERE THEY ARE INDICATED ON THE DRAWINGS. USE $1\frac{1}{2}$" LONG SEAM WELDS WHERE INDICATED. USE E60XX ELECTRODES.

G. LAP END JOINTS OVER STRUCTURAL SUPPORTS, 2" MIN.. PROVIDE SIDE LAPS AS REQUIRED BY THE DECKING TYPE.

H. WHERE ATTACHMENTS ARE OTHERWISE NOT INDICATED ON THE DRAWINGS, USE THE FOLLOWING MINIMUM ATTACHMENTS:

LOCATION	WELD
TRANSVERSE SUPPORTS	4 PUDDLE WELDS PER SHEET
PARALLEL SUPPORTS	PUDDLE WELDS @ 24"o.c.
SEAM ATTACHMENTS	BUTTON PUNCH @ 24"o.c.

I. SUBMIT QUALIFICATIONS AND CERTIFICATES FOR ALL WELDERS. SUBMIT WELDING PROCEDURES FOR APPROVAL BY THE ARCHITECT AND DSA.

J. SUBMIT SUBSTANTIATING DATA DEMONSTRATING CONFORMANCE TO THE SECTION PROPERTIES INDICATED ON THE DRAWINGS.

JOBSITE OBSERVATIONS:

COORDINATE WITH THE ARCHITECT AND THE DSA FIELD ENGINEER TO ARRANGE FOR THE FOLLOWING STRUCTURAL OBSERVATIONS, PER SECTION 1701A CCR:

A. FOUNDATION CONCRETE PRIOR TO THE FIRST POUR.
B. SLAB ON GRADE CONCRETE PRIOR TO FIRST SLAB POUR.
C. STRUCTURAL STEEL AND METAL DECK AFTER SUBSTANTIAL COMPLETION AND PRIOR TO CLOSING IN BY SUBSEQUENT CONSTRUCTION.

INSPECTION:

A. COORDINATE WITH THE INDEPENDENT INSPECTION AGENCY DESIGNATED BY THE ARCHITECT TO PERFORM THE INSPECTIONS LISTED BELOW:

SECTION	INSPECTION
SOILS	COMPETENCE OF SOILS - BY SOIL ENGINEERS
CONCRETE	PLACEMENT OF REINFORCEMENT PLACING OF CONCRETE, STRENGTH TESTS
H.S. BOLTS	INSTALLATION
WELDING: FIELD WELDS	ALL WELDS
METAL DECK	DIAPHRAGM WELDING
EXPANSION ANCHORS	INSTALLATION
EPOXY DOWELS	INSTALLATION

B. SPECIAL INSPECTION SHALL BE PROVIDED PER C.B.C. 1998 FOR THE FOLLOWING ITEMS:

1. CONCRETE TEST SPECIMEN
2. WELDING
3. EXPANSION BOLTS
4. HIGH STRENGTH BOLTING
5. SHOTCRETE

VOLUNTARY PARTIAL SEISMIC UPGRADE

A FULL SEISMIC UPGRADE OF THE BUILDINGS IS NOT INTENDED NOR REQUIRED.

THIS PROJECT CONSISTS OF THE FOLLOWING PARTIAL UPGRADE TO IMPROVE THE RESISTANCE OF SEISMIC LOADS IN THE BUILDINGS. THE WORK DOES NOT BRING THE BUILDINGS INTO FULL COMPLIANCE WITH THE CURRENT CODE. IT REPRESENTS THE ENGINEER OF RECORD'S JUDGMENT OF LIFE SAFETY HAZARDS ONLY.

BUILDING 18 - ADDED NEW CONCRETE SHEAR WALLS AND FOUNDATIONS
STRENGTHENED EXISTING FOUNDATIONS
PROVIDED NEW COLLECTORS

REVISION	DESCRIPTION	INITIAL	DATE
△1	ADDENDUM		9/15/03
	RECORD DRAWINGS		4/19/05

ISSUED FOR PROGRESS REVIEW	3/4/02
ISSUED FOR PLAN CHECK	4/15/02
ISSUED FOR PERMIT	7/02/02
ISSUED FOR BID	
ISSUED FOR CONSTRUCTION	

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PROJECT:

BUILDING 18 SEISMIC REHABILITATION

COLLEGE OF SAN MATEO
3401 CSM DRIVE
SAN MATEO, CA 94402

SAN MATEO CO. COMMUNITY COLLEGE DISTRICT

IDENTIFICATION STAMP
DIVISION OF THE STATE ARCHITECT

APPL 01-104536

AC_____ FLS_____ SS_____
DATE _____

SHEET TITLE:

GENERAL NOTES

PROJ. NO.	A95003.41
DRAWN BY	MAJ
DESIGNER	
PROJ. MGR.	
PEER REVIEW	

SHEET NUMBER:

S0.1

NUMBER: OF:

R & P JOB No. M0037.001
PRINCIPAL-IN-CHARGE RM
DRAWER No. M0F1
REMARKS:

M0F1S01.dwg 6-11-02 8:30:31 am